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# Cyclists in shared bus lanes: could there be unrecognised impacts on bus journey times?

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This paper contributes to debates around improving the modelling of cycles, through an exploratory case study of bus-cycle interactions in London. This case study examines undocumented delays to buses caused by high volumes of cyclists in bus lanes. It has generally been assumed that cyclists do not noticeably delay buses in shared lanes. However, in many contexts where cyclists routinely share bus lanes, cyclist numbers have historically been low. In some such places, bus lanes are now seeing very high volumes of cyclists, far above those previously studied. This may have implications for bus – and cycle – journey times, but traditionally traffic modelling has not represented the effects of such interactions well. With some manipulation of parameters taken from models of other cities, the model described here demonstrates that cycles can cause significant delays to buses in shared lanes, at high cycling volumes. These delays are likely to become substantially larger if London's cycling demographic becomes more diverse, because cyclist speeds will decline. Hence bus journey time benefits may derive from separating cycles from buses, where cycle flows are high. The project also suggests that microsimulation modelling software, as typically used, remains problematic for representing cyclists.

# 1. Introduction

Microsimulation modelling has substantially improved the ability to understand interactions in different traffic conditions. The approach represents vehicles individually, using distributions to model heterogeneous behaviour, such as differing levels of gap acceptance by drivers. These parameters can be adjusted to be specific to cities, countries or regions.

With increasing computing power, microsimulation can represent ever more complex traffic systems (Kara *et al.*, 2014; Wood, 2012). These new possibilities bring new challenges. Microsimulation, like other transport modelling methods, has been shaped by traditions of car dominance and car dependency (Naess *et al.*, 2014). As Kretz *et al.* (2013: p. 2) comment

[E]laborate methods for the planning of vehicular traffic in cities have been developed and are in perpetual use. With these tools often the vast majority of awareness and working hours are invested in vehicular traffic with the implicit assumption that cyclists and especially pedestrians always 'somehow' will make their way.

This has encouraged the development of specific pedestrian modules and dedicated pedestrian simulation models (Kretz *et al.*, 2013). Microsimulation modelling of cyclists is by contrast based on driver behaviour and vehicle characteristics, using different parameter distributions (e.g. speed). However, using default parameters, modelled cyclist behaviour may appear odd or unlikely: waiting behind cars in the middle of a congested lane, or failing to 'bunch up' with high numbers of

cyclists. Kohli *et al.* (2014) comment that there is a reduction in effective passenger car units (PCUs) per cyclist at times when there are higher cycling volumes. This is likely due to such behaviour, often not captured in traditional traffic models.

Some problems may be addressed by parameter adjustment, although often default parameters are used in modelling, if cyclists are included at all. A more fundamental issue remains: are cyclists basically similar to cars, or – like pedestrians – different in kind? The project described here explores these issues in modelling cyclist–bus interactions in one London location (London Bridge northbound carriageway), using PTV Vissim.

In countries with higher levels of cycling, such as Denmark and the Netherlands, bus lanes are not generally seen as cycle infrastructure, although cyclists may at times share space with buses, for example in city centres with restricted car access. By contrast bus lanes in the UK, which are mostly shared with cyclists as well as taxis and sometimes also motorcycles, have long been considered a core part of cycle and bus provision (Cycling England, 2010; although see also TfL, 2015b). Many argue (e.g. TRL, 2004) that shared bus lanes are win–win: cyclists benefit from reduced mixing with motor traffic, without delaying buses. The present paper explores the extent to which the second assumption is true when cycle flows are high. It has policy relevance for cities seeking to grow cycling, particularly among currently under-represented demographics, while supporting other sustainable modes (GLA, 2013).

Research focusing on cyclists in shared bus lanes is surprisingly rare. A study by TRL (2004) surveyed cyclists (largely men aged 25–39) using bus lanes at six sites in Edinburgh, Hull, Derby and London. It found bus lanes 'very popular' by comparison to typical local traffic conditions. To some extent, this is unsurprising. The survey targeted those choosing to use bus lanes; moreover 'typical traffic conditions' were likely to be poor. However, the study does suggest that, for current cyclists, in the UK often young-to-middle-aged men, bus lanes are seen as superior to mixing with general traffic. Wider restrictions on cyclists using bus lanes would cause a deterioration in cycling environments and probably also a reduction in cycling.

More recent research, however, suggests bus lanes are unlikely to encourage mass cycling among a wider demographic. Stated preference studies involving cyclists and non-cyclists (TfL, 2010, 2012) showed bus lanes are not viewed as preferable to painted advisory cycle lanes, although both are seen as better than nothing (see Figure 1). Both bus lanes and painted cycle lanes are seen as unsuitable for cycling with or by children (Aldred, 2015). Separated cycle infrastructure is substantially preferred by both existing and potential cyclists, particularly currently under-represented groups (Kohli *et al.*, 2014; TfL, 2010, 2012). Figure 1 illustrates the change in a nominal measure of 'utility' related to different types of infrastructure improvement, based on a Transport for London (TfL) study (TfL, 2010).

In London, the development of separated cycle infrastructure has been criticised as likely to cause bus delays, for example by London TravelWatch, an organisation that represents the capital's transport users (London TravelWatch, 2014). Underlying this claim is an assumption that the status quo has no impact on bus journey times, and therefore any reallocation of space and time towards cyclists can only be negative (or at best neutral) for bus journey times. This belief is expressed in much UK policy literature. Cycling England (2010: p. 2) design guidance on bus lanes and bus stops states

The primary factor affecting delay [to buses] is the width of the bus lane (A minimum of 4 m is suggested in the guidance to allow buses to pass cycles within the lane. However, given a 1 m dynamic envelope for a cycle, and a 2.5 m bus width, this would imply potentially a very close pass (0.5 m). Current TfL draft guidance on accessible bus stops cites 4.5 m). However, even with 3 m wide lanes, most cases of delay involve buses slowing down behind a cyclist before stopping at a bus stop. This is unlikely to have much effect on overall bus punctuality. Most cyclists try to avoid delaying buses, either by cycling faster or by allowing buses to pass.

This seems to be based on the TRL (2004) paper, where observers recorded impressions of whether buses and cycles delayed each other: their busiest route for cycling carried a maximum of 100 cyclists per peak hour. It would be difficult to use this method for much higher flows, yet an increasing number of UK towns and cities now experience far more than 100 cyclists per peak hour within shared bus lanes. Although authors now suggest that higher volumes may significantly delay buses and other traffic (Wedderburn, 2015: p. 12), little work has been done to quantify this (Kohli *et al.*, 2014).



Figure 1. Stated preferences for different types of cycle infrastructure among Londoners (source: adapted from TfL (2010))

If shared bus lanes at high cycling flows might substantially delay buses, separating the two would bring intrinsic bus journey time benefits that could – in theory – negate disbenefits due to changes in junction timing and/or reduction in dedicated bus space. The impact on buses of creating segregated space for cycles on or in parallel with bus routes would thus become something to be judged on a case-by-case basis. That would involve measuring current (and future) delays to buses caused by sharing with cycles, and examining whether these would be outweighed by any losses caused by reduction in space or time priority for buses. The case study presented here sketches out a simplified version of this approach.

# 2. Context

## 2.1 Context: London

The study used PTV Vissim (Version 8.00-05 (57518)) to produce a microsimulation model exploring the impact of cycles on bus journey times at one location in London. Vissim was used because of its successful application in COWI (2013) to model cyclist flows; meaning that the team had access to these non-standard parameters as a starting point.

Owing to interest in the impact of high cycling flows, and to keep the model simple (covering only a link section), the northbound carriageway of London Bridge was selected. The impact of cycles on buses at junctions may be different (see Kohli *et al.*, 2014); and relationships characterising London Bridge may not apply elsewhere. Therefore, the research is exploratory. It does, however, represent a first attempt to test the possible presence and establish the potential magnitude of delays to buses caused by high cycling flows in a London context. The approach is likely to be increasingly relevant in congested cities as cycling grows.

London has an excellent public transport system, with major recent investment in bus priority measures. It experiences congestion, particularly at peak times in inner and central areas, alongside ongoing population growth. A Department for Transport (DfT, 2004) report on London's early bus priority measures concluded that bus journey times had not substantially improved, but that without these measures, journey times would have lengthened further. Despite relatively limited impact on delays, bus priority measures did change perceptions of London's buses, improving reliability and achieving mode shift (TfL, 2009). London's mode shift towards sustainable modes continues (TfL, 2015a, 2015b). However, there are tensions between buses and cycling, with demands for segregated cycling infrastructure seen as causing problems for buses.

London has seen strong growth in cycling in recent years from a low base: cycling now carries as many people as do the city's Docklands Light Railway and taxis combined. In recent years, there has been a substantial uplift in investment (GLA, 2013), although lagging well behind investment in public transport modes. The cycling mode share is currently 2% for London overall, but on busy corridors – often Transport for London Road Network routes, which also form part of the city's Bus Priority Network – flows of 800 cycles per peak hour and more are now routinely recorded on key routes (TfL, 2013a). Figure 2 shows the London Bridge area in Central London context.

## 2.2 London Bridge case study

London Bridge is one of six bridges that both cyclists and motorists can use to cross the Thames between Westminster and the Tower of London. Morning peak hour flows on the northbound carriageway are around 2000 vehicles/h, over half being cycles. This northbound carriageway has one bus lane and two general motor traffic lanes, all of which are relatively narrow (c. 3 m).

Considering people flows, Table 1 shows that cyclists using the bridge easily exceed people travelling by car, taxi, light and heavy goods vehicle (HGV), and motorcycle combined. Figures provided by TfL for bus occupancy on this corridor during the morning (a.m.) peak are 27 passengers per bus. Normally, however, bus numbers would be approximately 100 based on scheduled services: this was lower (58 buses) during the data collection period due to roadworks in the area.

Hence Table 1 provides two estimates of bus occupancy; the first uses 27 passengers per bus and the second 47 passengers per bus (assuming a proportionally raised occupancy due to fewer buses serving the routes during the surveyed period). In the first case, a third of person-flow is made up of cycles and 42% bus passengers; in the second, a quarter of person-flow consists of cycle users and over half consists of bus passengers. The car/van/taxi occupancy figure of 1.2 has been taken from average English car or van occupancy rates for commuting and business trips, likely to dominate flows during peak hour (DfT, 2016). Motorcycle and HGV occupancy figures given by Banister (2008) were used.

# 3. Methods

# 3.1 Microsimulation of cyclists

Although microsimulation provides the ability to model road user behaviour accurately, most studies have not done this for cyclists (Twaddle *et al.*, 2014). As COWI (2013: p. 7) comments

During simulations of road traffic, cyclists and pedestrians are usually included to represent their effect on road capacity. [...] Whether the cyclists' behaviour is correctly represented is normally not considered, as they are not the primary focus.

But where cyclist volumes are high, modelling their behaviour unrealistically could lead to incorrect assumptions about capacity. Kohli *et al.* (2014) argue that PCU values for cyclists -a



Figure 2. London Bridge area. A full-colour version of this figure can be found on the ICE Virtual Library (www.icevirtuallibrary.com)

Table	1.	Vehicular and	person flow on	1 London Bridae	northbound a.	m. peak	(8 - 9)	a.m.)	)
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	Pedal cycle	Motorcycle	Bus	Light vehicle	Heavy vehicle	Total
Hourly vehicle total	1201	287	58	460	41	2047
Vehicles: % of total	59	14	3	23	2	100
Occupancy: persons/vehicle	1	1.1	27/47	1.2	2.3	_
People #1 (low bus estimate)	1201	316	1566	552	94	3729
People #2 (high bus estimate)	1201	316	2726	552	94	4889
People #2: % of total	25	6	56	11	2	100

key measure of capacity in traditional models such as Saturn and Transyt – vary depending on the road infrastructure. Where cycling flows are low, this may not matter; however, at high volumes, modelled capacity may be highly sensitive to assumptions about cyclist behaviour.

Vissim, like many microsimulation tools (e.g. Aimsum, Sumo), uses discrete time intervals and independently models longitudinal and lateral motion. The longitudinal approach is space continuous, utilising a 'car following model'. Lateral movement of motor vehicles is modelled using a discrete lane choice model, where position and speed of other road users and the desired route of the individual vehicle are taken into account in the lane-choosing process (Twaddle *et al.*, 2014: p. 141). For cyclists, such behaviour seems unrealistic, with platooning and under- and overtaking within a lane likely. But incorporating this natural feature of cyclist behaviour requires adjusting parameters because it is not 'natural' vehicle behaviour in existing models. Hence to include something resembling the typical behaviour of cyclists, it is not sufficient to use the parameters given in currently available modelling software. COWI (2013) used a continuous lateral access to model cyclist behaviour in Vissim, also used here, which provides a more realistic depiction of cyclist behaviour.

Different aspects of cyclist behaviour may present different levels of problem. Twaddle *et al.* (2014: p. 145) divide (potentially) modelled cyclist behaviour into three categories: operational, tactical and strategic. The strategic level is outside this project's remit, referring to 'planning the trip and selecting a route'. 'Operational' behaviour means 'automatic actions carried out by a bicyclist to control the bicycle and ride through the traffic environment'. 'Tactical' behaviour by contrast 'includes short term maneuvers that a bicyclist consciously selects to deal with the current traffic situation', such as swerving and deceleration to avoid collisions. Twaddle *et al.* (2014) conclude that, with calibration using field data, operational cycling behaviour (but not, perhaps, tactical behaviour) can be accurately modelled using software such as Vissim, although often this is not done.

#### Table 2. Key model parameters

Adjusted parameter	Initial parameters	Process of adjustment
Cyclist behaviour – speed distributions Cyclist behaviour – acceleration	COWI cyclist speed distributions	As described below, the distribution was iteratively shifted until the modelled bus journey times corresponded to those measured
and deceleration	distributions	No change
Cyclist behaviour – following parameters	COWI cyclist behaviour	Due to the high volume and higher speed, the minimum look-ahead distance was increased from 20 m to 40 m
Cyclist behaviour – overtaking parameters	COWI cyclist behaviour	Minimum headway (front/rear) was decreased due to observed cyclist behaviour at this site (0.5 m to $0.2$ m) and 'consider subsequent static routing decisions' was ticked
Bus driver behaviour – overtaking parameters	TfL Vissim template	Advanced merging behaviour turned on, as advised for new models

The process began by using parameters for traffic behaviour provided by TfL, which adjust default Vissim parameters to reflect London driver behaviour. Following this, the project's main starting point for model parameters was a recent study in Copenhagen, Denmark (COWI, 2013). The COWI study sought to represent the capacity and behaviour related to cyclists as accurately as possible. It involved large-scale data collection and analysis, followed by a lengthy technical process to create a far more accurate Vissim cyclist template. This provided an excellent starting point for the model. However, during calibration the COWI parameters had to be adjusted to represent London cyclist (and bus driver) behaviour.

The COWI study identified ten key parameters for the microsimulation of cyclists.

Basic parameters are

- vehicle characteristics
- speed distributions
- *acceleration (and deceleration) distribution.*

Parameters regarding bicycle paths are

- following parameters
- overtaking parameters
- behaviour at narrowing sections
- *behaviour at bus stops.*

Parameters regarding intersections are

- behaviour in waiting zones
- behaviour at stop lines
- behaviour at right turns.

For this study the five emphasised in italics are key. There were insufficient site data to look at different vehicle characteristics (i.e. types of cycles); in any case cyclists tended to be using standard cycles (i.e. not e.g. cargo cycles). The site did not have narrowing sections, waiting zones, stop lines, or right turns. 
 Table 3. Vissim bus journey time data: comparative effect on bus journey times

Scenario	Median journey time: s	Increase: %
No cyclists	22·4	
London cyclists	26·5	18
Copenhagen cyclists	32·6	46

All cycle settings were initially set to match COWI settings. An iterative process involved slowly adjusting settings to achieve validation against site-specific data. Another key element has been the behaviour of buses, and to a lesser degree general traffic, important to achieve a realistic understanding of the impact of cyclists on other vehicles.

#### 3.2 Parameter adjustment summary

Tables 2 and 3 summarise parameter adjustment, also highlighting differences between Vissim default settings, TfL 'London' settings, COWI cyclist settings, and the final settings used. Default and TfL do not refer to cyclists specifically, but to all vehicles on urban links. The COWI and multimodal/London Bridge settings are applied specifically and only to cyclists on those urban links, with general traffic/vehicles being controlled by the settings in the TfL behaviours.

The adjustment process is described in detail below in relation to cyclist speed, where the parameters used required substantial change. Further details of parameters used are in the Appendix, including more detail about the other four key parameters and how these were manipulated to provide greater realism than offered by the default parameters. For example, buses were allowed free lateral positioning so that they can overtake cyclists.

# 3.3 Network coding

The network was coded using the inbuilt mapping services within Vissim to create an accurately scaled background. As the focus was interaction between buses and cyclists in a bus lane, only the northbound movement across London Bridge



Figure 3. Study site (© OpenStreetMap contributors)



Figure 4. Example video capture image (camera facing south)

was modelled. This included the bus lane (nearside lane) and two general traffic lanes (middle and outside lanes). Figure 3 shows the extent of the study site.

# 3.4 Traffic counts and data collection

A comprehensive data collection exercise was carried out by PCC Traffic Information Consultancy during May 2015. This involved a series of video surveys, positioned at key intervals along London Bridge to capture northbound movement, which allowed the provision of detailed manual classified counts of vehicle totals in each lane, also capturing vehicle volumes for any traffic straddling lanes – that is while overtaking cyclists. Figure 4 shows an example video capture image during peak hour.

Travel time data were collected for all buses, between southern and northern bus stops. All data were collected over a 5 d period, to cover morning, midday and evening peaks. The a.m. peak (8.00–9.00) had significantly higher northbound traffic flow compared to the midday and p.m. peaks, particularly in terms of the number of cyclists (1201 compared to 95 and 347, respectively). Therefore, the morning was chosen as the most appropriate peak hour to model. Buses were entered at exact times throughout and cycles entered in 5 min volumes, based on the detailed surveys.

#### 3.5 Vissim model specification

The specification for the Vissim model is itemised below.

- Vissim version: 8.00-05.
- Testing year: 2015.
- Time period: a.m. peak period, 7.45–9.00 (including 15 min warm-up period).
- Evaluation period: a.m. peak, 8.00–9.00.
- Vehicle types defined: cars, HGVs, buses, motorcycles, pedestrians and cyclists.

Results evaluation method: models have each been run over five random seed profiles to reflect day-to-day variations in traffic patterns and profiles; the results have then either been averaged or used in full.

## 3.6 Speed distributions

After the initial calibration exercise, ensuring the correct traffic volumes were present, the model was validated using observed bus journey time data. Bus routing was modelled assuming buses stopped at one stop, but not both (as per video observations). During this process, data from five random seed runs was used each time to give a representative comparison to the 5 d worth of site data. The validation was assessed using standard criteria in the UK *Design Manual for Roads and Bridges.* These state that modelled journey times compared with observed journey times should be within 15% (or a minute, if higher) for 85% of the routes (DfT, 1996: p. 71).

The COWI profiles for cyclists' desired speeds proved inappropriate. When used, modelled bus journey times were substantially longer than the observed times, implying London Bridge cyclists are riding (or trying to ride) much faster than Copenhagen cyclists, even using downhill Copenhagen settings. An initial adjustment of the speed profiles was further refined, based on site observations of cyclists travelling across the segment in question, to give the final profile. Only 7% of London Bridge cyclists have a desired speed of under 19 miles/h (30 km/h), while this is true of almost all Copenhagen cyclists, even on downhill sections.

The speed distribution presented here represents a segment where both desired and actual speeds are likely to be relatively fast, compared to journey averages. People are cycling to or from work on a segment with no intersections, bends, crossings, parking or loading, and where the speed limit for motor traffic (with which they mix) is 30 miles/h (48 km/h). The platooning here means cyclists are effectively 'drafting' each other, reducing wind resistance. Indeed, London Strava data records riders obtaining achieved speeds of 20-25 miles/h (32-40 km/h) and even 25-30 miles/h (40-48 km/h) despite this covering a longer London Bridge segment, including a junction likely to slow cyclists down (Strava, 2017a, 2017b). The academic literature on achieved cycling speed tends: (a) to focus on whole-journey speeds (e.g. El-Geneidy et al., 2007), which will be much lower than peak segment speeds, or (b) to focus on speeds through intersections (e.g. Pein, 1997), which will be relatively low, or areas where there is much more interaction with pedestrians and/or vehicles pulling in and out (e.g. Davies et al., 2003; Singh, 2012).

The high desired and actual cyclist speeds (Figures 5 and 6) calculated across this section do not imply whole-journey speeds are much faster in London than Copenhagen. That will depend on factors such as traffic light phasing; and



**Figure 5.** Comparison of three cyclist speed profiles (Vissim desired speed parameters)



Figure 6. Achieved compared with desired speeds at baseline, modelled cyclists on London Bridge

Copenhagen has introduced 'green waves' for cyclists to reduce the time they spend stopped at red lights. However, maintaining or even attaining a speed of 20 miles/h is likely to be challenging for many potential riders, especially women, whose speeds tend to be lower than men's (Aldred and Crosweller, 2015).

It should be noted that many cyclists did not attain their desired speed over the bridge. Specifically, 55% of cyclists had a desired speed of 20 miles/h or less, while 73% travelled through this section at 20 miles/h or less. Given the conditions here (relatively conducive to high speeds – no side roads, car parking, or junctions) this may indicate the potential of buses to delay cycles, although this is not the focus here.

Figures 7 and 8 demonstrate the difference when using the bespoke and COWI profiles. Using the COWI level desired speed profile, only 51% of bus journeys validated correctly. Using the

'London Bridge' profile, 85% of bus journey times fall within the surveyed range. Figures 7 and 8 illustrate the two profiles in comparison to recorded bus journey times between 8 and 9 a.m.



Figure 7. Vissim bus journey time data: COWI level path setting





It should be noted that the model is relatively simple and is unable, for example, to include the split cycle and offset optimization technique split cycle and offset optimization technique (Scoot) settings used across the London network, which aims to improve network resilience to delays by re-timing signals when needed. This may contribute to the appearance of occasional very high bus journey times within the model, particularly towards the end of the peak hour, whether modelling baseline or alternative scenarios. Hence median rather than mean journey times are used here to illustrate delay. The use of means would skew the modelled delays upwards because of these occasional anomalous results; whereas the modelled and actual baselines were much closer in terms of medians.

Figures 9 and 10 illustrate how much closer the medians are than the means, for modelled and actual baseline data. However, the medians remain systematically high, albeit to a much lesser extent. This reflects the exploratory nature of the



**Figure 9.** Actual and modelled baseline data: comparison of means, 10 min intervals



Figure 10. Actual and modelled baseline data: comparison of medians, 10 min intervals

model and the limited research backing for the parameters and underlying assumptions. A measurable delay exists, but this small-scale piece of work was unable to capture it entirely accurately. The paper returns to this point in discussing the need for further research and data collection in this area to support better modelling.

# 4. Findings

Once the calibrated model achieved a suitable fit between modelled and observed bus journey time data, results were compiled to measure any delays buses experienced travelling northbound along the stretch of bus lane on London Bridge. Delay was measured as the increase in journey time comparative to a sample scenario run with no cyclists. Journey times were measured between bus stops on either side of the bridge, sufficiently away from the bus cages that buses stopping to board/alight passengers were not included in any delay calculation. General traffic had only a minimal effect on bus delays in the 'no cyclists' scenario, being mostly limited to the outside lane, even during the busiest morning peak.

Table 3 shows median bus journey times for each scenario at peak hour, along with the percentage increase over the 'no cyclist' model (representing the amount of delay caused by the flow of cyclists). Current volumes of London cyclists are associated with a median 18% increase in peak hour bus journey times, while the same volumes of Copenhagen cyclists (riding as per the COWI level profile) would be associated with a larger increase of 46% over baseline.

The bus journey times for each scenario at peak hour have been compared with each other using SPSS statistical software (Mann–Whitney non-parametric tests, which do not require a normal distribution) and the three scenarios were all significantly different from each other (p < 0.001).

Running the model with 100 cyclists per hour (the 'very busy' case included in the TRL (2004) study), median peak hour bus journey times over London Bridge only increase by 2.2% compared to the modelled baseline. This is indeed relatively negligible and hence it is unsurprising that the TRL observers detected no delays at those cycling volumes.

# 5. Discussion

# 5.1 Limitations of this study

This has been a small-scale study with many limitations. It is not possible to draw broader conclusions about specific delays one might expect at particular cycling volumes, which must be investigated on a case-by-case basis. The paper has, however, made the case for such investigation, where shared bus lanes are present or being considered, and current, expected and/or desired cycle flows are high. Shared bus lanes may delay buses in such contexts, perhaps more so than separated provision.

Another limitation potentially relates to the possible impact of changes in speed limits. The speed limit (and hence desired speeds for buses) on London Bridge is currently 30 miles/h; however, this may be reduced. Would a lower speed limit therefore cut bus delays, by bringing desired bus and cycle speeds closer? Currently, modelled average bus speed along London Bridge is 20.0 miles/h at peak, excluding the reduction in speed caused by stopping at one of the two bus stops. (This was higher than the recorded speeds obtained for London Bridge from TfL by way of the iBus system; however, the latter included the acceleration or deceleration period when leaving or entering a bus stop.) Around half the cyclists have at present a desired speed below 20 miles/h, retaining potential to delay buses. For the two COWI speed profiles, representative of the slower cyclist speeds one might expect with a more diverse cohort, the medians would be around 17 miles/h (27 km/h) and 14 miles/h (22.5 km/h), with nearly all having a desired speed below 20 miles/h.

One final issue relates to black cabs, which in London are generally allowed to use bus lanes. These were not included in the model; nor were motorcycles, which can use TfL route network bus lanes. However, virtually no such vehicles used the bus lane during the morning peak, it being full of bicycles and buses, while the adjacent general motor traffic lane was almost empty. A different situation would have different implications for bus journey times. Moreover, reducing general motor traffic capacity to provide for cyclists could mean more taxis and motorcycles using the bus lane, unless restrictions were introduced.

#### 5.2 Summary of findings

The modelling indicates that buses travelling northbound along London Bridge are impacted significantly by cyclists at current peak flows, with an 18% median increase in journey time at peak hour along this route segment, compared to there being no cyclists present. The route segment is very short and simple, containing for instance no intersections, so this result cannot be extrapolated across the network. However, the project highlights the importance of accurately modelling cyclists in existing and proposed schemes, where baseline and/or options involve shared road space between these and other vehicles, to more accurately gauge vehicular delay. It also suggests that separation of bus and cycle flows may have the potential to reduce bus delays.

## 5.3 Implications for modelling

Modelling cycles accurately can have substantial implications for understanding delays and effective highway capacity. Using standard capacity assumptions (TfL, 2013a, 2013b) the carrying capacity of the northbound bus lane might be approximately 750 vehicles. A bus lane carrying 58 buses and 2232 cycles (0·2 PCU each) could then on paper still be safely under capacity. (A TfL road network route would additionally carry taxis and motorcycles, although this did not substantially affect bus operation in this case study.) However, the model shows substantial delays, illustrating the need to consider bus-cycle interactions at a granular level. This supports, and is supported by, the findings of Carrignon (2009) that a 0.2 PCU measure for cycles in mixed traffic is: (*a*) too low, given lane width below 4 m and (*b*) variable, with two-wheelers reaching saturation at 10% of flow.

The study highlights the importance of comprehensive site collected data. Here a significant amount of data was collected for traffic flows by vehicle type/lane and bus journey times. However, a more comprehensive previous study (COWI, 2013) was needed to help determine key cyclist specific parameters. These included speed profiles, acceleration/deceleration rates and behaviour-specific parameters (e.g. look-ahead distances, lateral behaviour) not available for London Bridge. Cyclists travelling across London Bridge are more aggressive and faster than those in Copenhagen, reinforcing the need for site-specific data. However, in low-cycling contexts modellers should consider how cyclist behaviour might change as cycling volumes increase. One likely reason for lower speeds of Copenhagen cyclists is the more equal gender split. Where an increase in cycling is being modelled, speed parameters from higher cycling contexts such as Copenhagen may be relevant.

Further work could examine how sharing space with motor vehicles affects cyclist journey times. Anecdotal evidence suggests that, in congested conditions, this can be substantial and encourage undesirable behaviour such as footway cycling. However, again this area is poorly understood. The study also suggests the utility of exploring ways of modelling cyclist behaviour separately from motor vehicles. Although the model was successfully parameterised, and provided useful information in an area with relatively little prior research, it highlighted the limitations of using driver behaviour as a model for cycling. Related problems have been observed for modelling shared space interactions between drivers and pedestrians (Gibb, 2015). This suggests the need to reconsider how to model the contexts in which road users interact, as well as the behaviour of the road users concerned.

#### 5.4 Implications for policy

Tentative policy conclusions may be drawn. It seems likely that where cyclist volumes are very high (substantially more than the 100 per hour defined as 'high' in TRL (2004)) this causes delays to buses. Such delays are at present neither well understood nor included in modelling work. Yet in London and in some other towns and cities in the UK, these volumes are being seen at peak hours on key bus routes, while current trends and targets suggest even higher cycling volumes will be experienced in the future.

Hence, in principle, reducing the numbers of cyclists in bus lanes through alternative provision might help reduce bus delays. There will be a trade-off involved if, for example, buses are then delayed at junctions due to the need to accommodate cyclists separately. However, this would need to be considered

empirically from case to case. Planners should be aware that where high cyclist volumes are experienced or predicted, shared bus lanes may cause bus delays that remain unrepresented in standard modelling approaches. While the bus lane modelled here is 3 m, even 4.5 m lanes (as found in some London streets) might create some delays, because drivers would be unwilling to overtake cyclists close to stops. Moreover, given a bicycle 'envelope' of 0.75 m, and a recommended passing distance of 1.5 (as currently used in campaigns by a number of English police services, such as 'Give space, be safe'), a 2.5 m wide bus still cannot necessarily safely overtake a cyclist where there is a vehicle in an adjacent lane.

There are additional reasons for not mixing buses and cycles, particularly where high cycling flows exist or are desired. First, mixing with buses can be frightening for cyclists, with dangerous overtaking or bicycle-following behaviours potentially common (De Ceunynck *et al.*, 2015). Second, cyclists and potential cyclists see even wide bus lanes, even those with 20 miles/h speed limits, as substantially inferior to separate provision (Aldred, 2015). Where there is space for 4.5 m bus lanes, it may be possible to provide at least light segregation alongside narrower bus lanes. Sharing is particularly problematic where children are involved (Aldred, 2015). However, this paper is distinctive in highlighting that such separation can potentially positively impact bus passengers and bus operations.

Removing or reducing high volumes of cyclists from bus lanes could be done in different ways. It is impractical to ban cyclists from bus lanes in conditions of high cycling flows, and would lead to substantial delays for, and conflicts with, general motor traffic. Two options are described below.

(a) The creation of separated cycle tracks, thus removing cyclists from bus lanes (although in the UK, cyclists would still be permitted to use the bus lanes, few will do so with tracks present). This would in principle substantially reduce bus delays where cyclist volumes are high. It will require taking space from other users: whether general traffic, buses, or pedestrians, or a combination. Sustainable transport goals would suggest this space should be taken from general traffic. However, in practice there might be some disbenefit for buses, which would have to be balanced against the reduction in delays caused by the separation of modes.

Separated tracks are increasingly being implemented on main roads in UK cities. They will, to be consistent, involve routing cyclists behind bus stops, so bus passengers need to cross the cycle track to board or alight from a bus. This entails decisions about whether pedestrians or cyclists should be given priority at such points. In the UK, where bus stop 'bypasses' and 'boarders' are still relatively rare, different arrangements are being tested. If bus and passenger flows are low, cyclist priority may be appropriate, but in contexts with higher pedestrian flows, pedestrian priority may be needed. Although cycle–pedestrian conflicts are low risk compared to cycle–bus or bus–pedestrian conflicts, designs must be mindful of the comfort of more vulnerable passengers.

(b) The creation of parallel routes that prioritise cyclists, so that they are less likely to use bus lanes. There are examples of this approach in London; for example, the Tavistock–Torrington Place cycle tracks in Camden are an alternative to the very busy Euston Road (a bus route). Parallel routes must be of high quality and nearby to attract cyclists; they must link useful destinations without excessive detour and provide adequate capacity to cater for predicted/desired flows, or cyclists will continue to use bus lanes.

Where cyclists currently routinely share space with buses, policy-makers should consider developing a matrix to guide decisions about mixing or separation. It is recommended here that such a matrix should use desired rather than current cycling and bus flows, because shared bus lanes are a relatively unattractive provision for cycling, particularly by underrepresented groups such as women, older people and children. The planning process should aim to provide networks of both cycle and bus routes that are sufficiently high in quality and dense to attract mass custom, given the high efficiency of both modes. Decisions may then be taken to re-route cycle or bus networks to provide better for both modes (Figure 11).

# 6. Conclusions

Planners may be wrong if they assume cyclists do not delay buses in shared lanes. At high cycling volumes, delays may be



Figure 11. Example matrix of provision

substantial and may increase in a non-linear fashion, particularly if cycling becomes more demographically diverse. Hence, creating separated space for cyclists does not necessarily detract from bus provision. It may even improve it, if space is taken from private cars. In this case, there may be a double benefit: buses would benefit directly from no longer being delayed by cycles, and would become relatively more attractive compared to the car.

The extent or existence of any benefit for buses must be determined on a case-by-case basis: this paper has only shown the principle is valid. Efforts to improve the modelling of cyclists are also recommended: current microsimulation packages are limited in their ability to represent actual cycling behaviour. For this relatively simple project, manipulation of parameters was sufficient to represent cyclist behaviour. However, substantial improvements need to be made to modelling to enable a more realistic depiction of cycling, and hence better predict outcomes of interest.

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## Appendix: Vissim parameters used

The following provides more detail on the parameters altered from either the London default settings or those used in the COWI study.

Other than at bus stops, all buses use a behaviour named 'Vehicles overtaking', with the following parameters – the highlighted parameters are those which have been changed from standard urban driver behaviour (see Figures 12-14).

Other than at bus stops, all cyclists use a behaviour named 'Multimodal settings', with the following parameters – the highlighted parameters are those which have been changed from standard urban driver/cyclist behaviour (see Figures 15–17).

At bus stops, cyclists use a behaviour named 'Multimodal settings – bus stop' if there is a bus present – the highlighted parameters are those which have been changed from 'Multimodal settings' (see Figures 18 and 19).



Figure 12. Vissim driver behaviour: vehicles overtaking - following

: 101 Name: Vehicles overtaki	ing
lowing Lane Change Lateral Signal Contr	rol
eneral behavior: Free lane selection	~
ecessary lane change (route)	
Own	Trailing vehicle
laximum deceleration: -4.00 m/s2	-3.00 m/s2
1 m/s2 per distance: 100.00 m	100.00 m
ccepted deceleration: -1.00 m/s2	-1.00 m/s2
aiting time before diffusion:	60.00 s Overtake reduced speed areas
lin. headway (front/rear):	0.50 m 🖌 Advanced merging
o slower lane if collision time is above.	11.00 s Consider subsequent static routing
afety distance reduction factor:	0.60 decisions
laximum deceleration for cooperative braking:	-3.00 m/s2
] Cooperative lane change	
laximum speed difference: 6.71 mph	
laximum collision time: 10.00 s	
] Lateral correction of rear end position	
laximum speed: 1.86 mph	
ctive during time period from 1.00 s un	til 10.00 s after lane change start

Figure 13. Vissim driver behaviour: vehicles overtaking - lane change

ollowing Lar	ne Change Li	ateral Signal Contr	rol		_
Desired positio	on at free flow:	Left			~
Keep latera	al distance to v	ehicles on next lane	(s)		
Diamond s	haped queuin	9			
Consider n	ext turning dir	rection			
Collision time	gain:	15.00 s			
Minimum long	gitudinal speed	i: 10.00 n	nph		
lime between	direction char	nges: 10.00 s			
Default behavi	ior when overt	aking vehicles on th	e same lane		
Overtake on sa	ame lane	Minimum lateral di	stance		
On left		Distance standing:	0.30 m bei 0 m	mph	
On right		Distance driving:	0.50 m bei 30	mph	
Exceptions for	overtaking ve	hicles of the followin	ng vehicle classes		
Count: 2 Veh(	Class	OvtL	OvtR	LatDistStand	LatDistDriv
1 60: C	yclist	<b>v</b>		0.20	0.30
2 70 M	lotorbike	V		0.20	0.30

Figure 14. Vissim driver behaviour: vehicles overtaking - lateral

lo.: 1000 Name: Cyclists -	Multimodal Settings	
In the second se	A Control  Car following model  Wiedemann 99  Model parameters  CC0 (Standstill Distance): CC1 (Headway Time): CC1 (Headway Time): CC2 ('Following' Variation): CC3 (Threshold for Entering 'Following'): CC4 (Negative 'Following' Threshold): CC5 (Positive 'Following' Threshold): CC5 (Speed dependency of Oscillation): CC7 (Oscillation Acceleration): CC7 (Oscillation Acceleration): CC7 (CC7 (Oscillation Acceleration): CC7 (CC7 (Coscillation Acceleration): CC7 (CC7 (CC7 (Coscillation Acc	~
<ul> <li>✓ Smooth closeup behavior</li> <li>□ Standstill distance for 1.00 m</li> <li>static obstacles:</li> </ul>	CC9 (Acceleration with 50 mph): 0.01 m/s2	

Figure 15. Vissim driver behaviour: multimodal settings – following

.: 1000	Name: Cyclists - Mu	ultimodal Settings	
llowing Lane Chan	ge Lateral Signal C	ontrol	
eneral behavior: F	ree lane selection		~
lecessary lane chang	e (route)		
	Own	Trailing vehicle	
laximum deceleratio	n: -4.00 m/s2	-3.00 m/s2	
1 m/s2 per distance:	100.00 m	100.00 m	
ccepted deceleration	n: -1.00 m/s2	-1.00 m/s2	
/aiting time before d	iffusion:	60.00 s	Overtake reduced speed areas
lin. headway (front/r	ear):	0.20 m	Advanced merging
o slower lane if collis	ion time is above.	11.00 s	Consider subsequent static routing
afety distance reduct	ion factor:	0.60	decisions
laximum deceleratio	n for cooperative braki	ng: -3.00 m/s2	
Cooperative lane c	hange		
laximum speed diffe	rence: 6.71 m	ph	
laximum collision tin	ne: 10.00 s		
Lateral correction	of rear end position		
laximum speed:	1.86 mph		
ctive during time pe	riod from 1.00 s	until 10.00 s after	lane change start

Figure 16. Vissim driver behaviour: multimodal settings – lane change

	me: Cyclists - Multi	modal Settings		
ollowing Lane Change	Lateral Signal Con	trol		
Desired position at free flo	w: Left			$\sim$
Keep lateral distance to	o vehicles on next lan	e(s)		
Diamond shaped queu	ing			
Consider next turning	direction			
Collision time gain:	15.00	c .		
Minimum Inneitudiant and	6.15			
viinimum iongitudinai spe	ed: 0.15	mpn		
lime between direction ch	hanges: 5.00	s		
Default behavior when ow	and the second find as a set			
belault beliavior when ov	ertaking vehicles on ti	he same lane		
Overtake on same lane	Minimum lateral c	he same lane		
Overtake on same lane	Minimum lateral of Distance standing	he same lane distance g: 0.20 m bei 0	mph	
Overtake on same lane On left On left	Minimum lateral o Distance standing Distance driving:	he same lane distance p: 0.20 m bei 0 0.33 m bei 30	mph ) mph	
Dvertake on same lane ☐ On left ☑ On right	Minimum lateral c Distance standing Distance driving:	he same lane distance p: 0.20 m bei 0 0.33 m bei 30	mph D mph	
Overtake on same lane ☐ On left ☑ On right Exceptions for overtaking	Minimum lateral of Distance standing Distance driving: vehicles of the follow	he same lane distance p: 0.20 m bei 0 0.33 m bei 30 ing vehicle classes	mph 0 mph	
Overtake on same lane ☐ On left ☑ On right Exceptions for overtaking Count: 6 VehClass	Minimum lateral of Distance standing Distance driving: vehicles of the follow OvtL	he same lane distance (c) 0.20 m bei 0 (c) 0.33 m bei 31 ing vehicle classes OvtR	mph 0 mph LatDistStand	LatDistDriv
Dvertake on same lane Overtake on same lane On left On left On right Count: 6 VehClass 1 10: Car 2 20: HGV	A minimum lateral c Distance standing Distance driving: vehicles of the follow OvtL	he same lane distance p. 0.20 m bei 0 0.33 m bei 30 inq vehicle classes OvtR	mph D mph LatDistStand 0.20 0.20	LatDistDriv 0.33
Overtake on same lane Overtake on same lane On left On right Exceptions for overtaking Count: 6 VehClass 1 10: Car 2 20: HGV 3 30: SD Bus	Vehicles of the follow Vehicles of the follow Vehicles of the follow OvtL	he same lane distance p: 0.20 m bei 0 0.33 m bei 30 ing vehicle classes OvtR	mph D mph LatDistStand 0.20 0.20	LatDistDriv 0.33 0.50
Count: 6 Overtake on same lane On left On right Exceptions for overtaking Count: 6 VehClass 1 10: Car 2 20: HGV 3 30: SD Bus 4 60: Cvrlist	Vehicles of the follow Vehicles of the follow OvtL	he same lane distance p: 0.20 m bei 0 0.33 m bei 30 ing vehicle classes OvtR V	mph D mph LatDistStand 0.20 0.20 0.10	LatDistDriv 0.33 0.50 0.50 0.10
Overtake on same lane ☐ On left ☑ On right Exceptions for overtaking Count: 6 VehClass 1 10: Car 2 20: HGV 3 30: SD Bus 4 60: Cyclist 5 70: Whotorbike	Vehicles of the follow	he same lane distance (0.20) m bei 0 0.33 m bei 30 ing vehicle classes OvtR V V V V V V	mph D mph LatDistStand 0.20 0.20 0.20 0.10 0.20	LatDistDriv 0.33 0.50 0.50 0.10 0.33
Overtake on same lane ☐ On left ☑ On right Exceptions for overtaking Count: 6 VehClass 1 10: Car 2 20: HGV 3 30: SD Bus 4 60: Cyclist 5 70: Motorbike 6 90: DD Bus	Vehicles of the follow vehicles of the follow OvtL	he same lane distance p: 0.20 m bei 0 0.33 m bei 30 ing vehicle classes OvtR Ø Ø Ø Ø Ø Ø Ø Ø Ø	mph D mph LatDistStand 0.20 0.20 0.20 0.10 0.20 0.20	LatDistDriv 0.33 0.50 0.50 0.10 0.33 0.50
Count de Denardo Hinter Or Dvertake on same lane ☐ On left ☑ On right Count: 6 VehClass 1 10: Car 2 20: HGV 3 30: SD Bus 4 60: Cyclist 5 70: Motorbike 6 90: DD Bus	Vehicles of the follow	istance istance is 0.20 m bei 0 0.33 m bei 30 ing vehicle classes OvtR V V V V V V V V V V V V V	mph D mph LatDistStand 0.20 0.20 0.20 0.10 0.20 0.20	LatDistDriv 0.33 0.50 0.50 0.10 0.33 0.50
Overtake on same lane Overtake on same lane Overtake on same lane On left On left Oright Count: 6 VehClass 1 10: Car 2 20: HGV 3 30: SD Bus 4 60: Cyclist 5 70: Motorbike 6 90: DD Bus	Vehicles of the follow over th	istance istance istance 0.20 m bei 0 0.33 m bei 30 ing vehicle classes OvtR V V V V V V V V V V	mph 0 mph LatDistStand 0.20 0.20 0.20 0.20 0.20 0.20	LatDistDriv 0.33 0.50 0.50 0.10 0.33 0.50

Figure 17. Vissim driver behaviour: multimodal settings – lateral

Name: Cyclists - Multi	modal Settings - Bus Stop	_
Ilowing Lane Change Lateral Signal Con	trol	
eneral behavior: Free lane selection	~	
ecessary lane change (route)		
Own	Trailing vehicle	
laximum deceleration: -4.00 m/s2	-3.00 m/s2	
1 m/s2 per distance: 100.00 m	100.00 m	
ccepted deceleration: -1.00 m/s2	-1.00 m/s2	
aiting time before diffusion:	60.00 s Overtake reduced speed areas	
lin. headway (front/rear):	0.20 m 🗹 Advanced merging	
o slower lane if collision time is above.	11.00 s Consider subsequent static routing	
afety distance reduction factor:	0.60 decisions	
laximum deceleration for cooperative braking:	: -3.00 m/s2	
Cooperative lane change		
laximum speed difference: 6.71 mph		
laximum collision time: 10.00 s		
Lateral correction of rear end position		
taximum speed: 1.00 mpn	10.00	
ctive during time period from 1.00 s ur	ntil 10.00 s after lane change start	

Figure 18. Vissim driver behaviour: multimodal settings – bus stop – lane change

0	Ni	ame:	Cyclists - Mu	ltimodal	Settings - Bus St	op		
ollowing	Lane Change	Late	ral Signal Co	ontrol				
Desired	position at free flo	ow:	Right				~	
🗸 Keep	lateral distance t	o vehi	icles on next l	ane(s)				
🗸 Diam	ond shaped que	uing						
	ider next turning	direct	ion					
Collision	time gain:		15.00	0 <				
	the goint		6.1					
Minimur	n longitudinal sp	eea:	0.1	o mpn				
Time bet	ween direction c	hange	s: 5.00	0 5				
	ancen ancedon e	-						
Default I	pehavior when ov	ertaki	ng vehicles or	n the sam	ne lane			
Default I Overtake	pehavior when ov e on same lane	vertaki M	ng vehicles or linimum latera	n the sam	e lane			
Default I Overtake	pehavior when ov e on same lane	vertakii M Di	ng vehicles or linimum latera istance standii	n the sam al distance	e 0 m bei 0	mph		
Default I Overtake	pehavior when ov e on same lane eft	vertakin M Di Di	ng vehicles or inimum latera istance standi	n the sam al distance ng: 0.20	ne lane e 0 m bei 0 3 m bei 3	mph ) mph		
Default I Overtake	pehavior when ov e on same lane eft ight	vertakii M Di Di	ng vehicles or linimum latera istance standii istance driving	n the sam al distance ng: 0.20 g: 0.33	e 0 m bei 0 3 m bei 30	mph ) mph		
Default I Overtake On le On ri Exceptio	pehavior when ov e on same lane eft ight ins for overtaking	vertakin M Di Di vehic	ng vehicles or linimum latera istance standii istance driving les of the follo	n the sam al distance ng: 0.20 g: 0.33 pwing vel	e 0 m bei 0 3 m bei 3 hicle classes	mph ) mph	LatDictDri	
Default I Overtake On le On ri Exceptio Count: 6	behavior when ov e on same lane eft ight ins for overtaking VehClass	vertakin M Di Di vehic	ng vehicles or linimum latera istance standii istance driving les of the follo OvtL	n the sam al distance ng: 0.20 g: 0.33 powing vel	e m bei 0 m bei 0 m bei 3 m bei 3 hicle classes OvtR	mph ) mph LatDistStand	LatDistDrin	v 0.20
Default I Overtake On le On ri Exceptio Count: 6 1 2	behavior when ov e on same lane eft ight vehClass 10: Car 20: HGV	vertakin M Di Di vehic	ng vehicles or iinimum latera istance standii istance driving les of the follo OvtL	n the sam al distance ng: 0.20 g: 0.32 powing vel	e ane e of the lane of the lan	mph ) mph LatDistStand 0.	LatDistDrin 10	0.20
Default I Overtake On le On ri Exceptio Count: 6 1 2 3	en anceaure a pehavior when ov e on same lane eft ight ms for overtaking VehClass 10: Car 20: HGV 30: SD Bus	vertakii M Di Di vehic	ng vehicles or iinimum latera istance standii istance driving les of the follo OvtL	n the sam al distance ng: 0.20 g: 0.32 powing vel	e lane 0 m bei 0 3 m bei 3 hicle classes OvtR V V	mph ) mph LatDistStand 0, 0, 0,	LatDistDriv 10	v 0.20 0.20 0.20
Default B Overtake On le On ri Exceptio Count: 6 1 2 3 4	e on same lane eft ght so for overtaking VehClass 10: Car 20: HGV 30: SD Bus 60: Cyclist	vertakin M Di Di vehic	ng vehicles or linimum latera istance standi istance driving les of the follo OvtL	n the sam al distance ng: 0.20 g: 0.33	e lane e 0 m bei 0 3 m bei 3 hicle classes OvtR V V	mph ) mph LatDistStand 0. 0. 0. 0.	LatDistDriv 10 10 10	v 0.20 0.20 0.20 0.20 0.10
Default I Overtake On le On ri Exceptio Count: 6 1 2 3 4 5	echavior when ov e on same lane eft ght vehClass 10: Car 20: HGV 30: SD Bus 60: Cyclist 70: Motorbike	vertakin M Di Di vehic	ng vehicles or iinimum latera istance standi istance driving les of the follo OvtL	n the sam al distance ng: 0.2( g: 0.3) pwing vel	e lane e 0 m bei 0 3 m bei 3 hicle classes OvtR V V V	mph ) mph LatDistStand 0. 0. 0. 0. 0. 0. 0. 0.	LatDistDriv 10 10 10 10	0.20 0.20 0.20 0.10 0.10
Default I Overtake On le Exceptio Count: 6 1 2 3 4 5 6	vehicle uncease of the openavior when over e on same lane eff ight ins for overtaking VehClass 10: Car 20: HGV 30: SD Bus 60: Cyclist 70: Motorbike 90: DD Bus	vertakii M Di Di vehic	ng vehicles or inimum latera istance standii istance driving les of the follo OvtL	n the sam al distance ng: 0.20 g: 0.33 pwing vel	ne lane e 0 m bei 0 3 m bei 3 m bei 4 M bei 4 M bei 4 M bei 0 M bei 0 M bei 0 M bei 0 M bei 0 M bei 0 M bei 1 M bei 3 M bei 4 M bei	mph 0 mph 2 LatDistStand 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.	LatDistDrin 10 10 10 10 10	0.20 0.20 0.20 0.10 0.10 0.20
Default I Overtake On re Exceptio Count: 6 1 2 3 4 5 6	behavior when ov e on same lane eft ight vehClass 10: Car 20: HGV 30: SD Bus 60: Cyclist 70: Motorbike 90: DD Bus	vertakin M Di Di vehic	ng vehicles or inimum latera istance standii istance driving les of the follo OvtL	an the sam al distance ng: 0.20 g: 0.32 powing vel	e ane ane e 0 m bei 0 3 m bei 3 hicle classes OvtR V V V V V V	mph D mph LatDistStand 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.	LatDistDrin 10 10 10 10 10 10 10	0.20 0.20 0.10 0.10 0.20
Default I Overtake On re Exceptio Count: 6 1 2 3 4 5 6	vector vincea or expension of the operation of the operat	vertakii M Di Di v vehic	ng vehicles or inimum latera istance standii istance driving les of the follo OvtL	an the sam	e ane e 0 m bei 0 3 m bei 3 hicle classes OvtR V V V V V V V	mph ) mph LatDistStand 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.	LatDistDriv 10 10 10 10 10 10	0.20 0.20 0.20 0.10 0.10 0.20

Figure 19. Vissim driver behaviour: multimodal settings – bus stop – lateral

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